10th World Congress of the European Water Resources Association on Water Resources and Environment "Panta Rhei" Athens, Greece, 5-9 July 2017

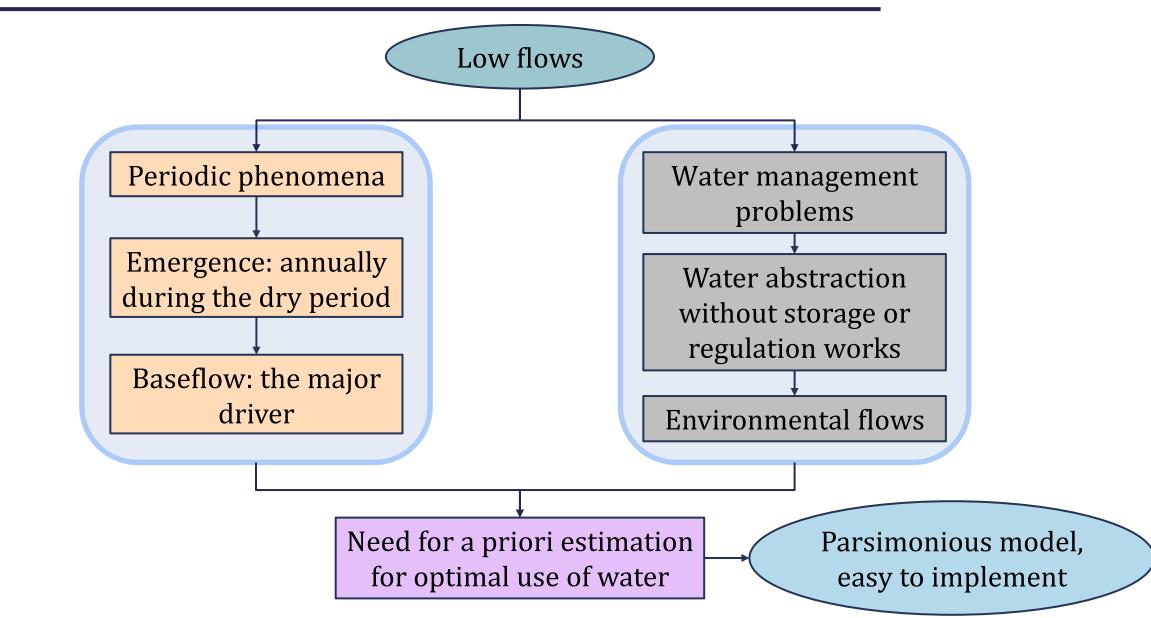
A simple model for low flow forecasting in Mediterranean streams

<u>Konstantina Risva ⁽¹⁾</u>, Dionysios Nikolopoulos⁽²⁾, Andreas Efstratiadis⁽²⁾, Ioannis Nalbantis ⁽¹⁾

(1) School of Rural and Surveying Engineering,, National Technical University of Athens, Greece

(2) School of Civil Engineering, National Technical University of Athens, Greece

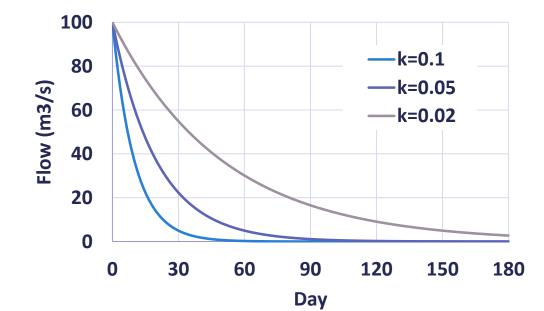
Low flows and water management

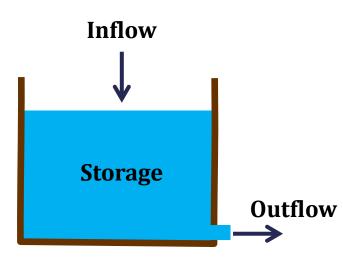


Key assumptions and definitions

- Rationale: Baseflow is the key driver of low flows during dry periods, represented as outflow through a linear reservoir.
- Modeling scheme: The low flow during the dry-period of a specific year *j* is modelled by an exponential decay:

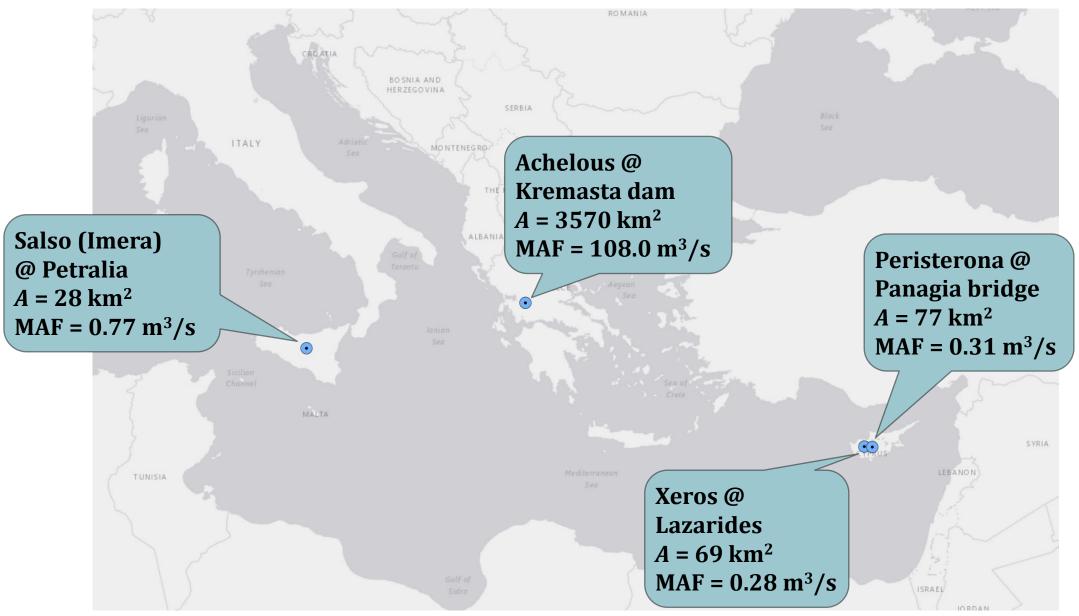
$$q_{jt} = q_{0j} \exp(-k_j t) \tag{1}$$





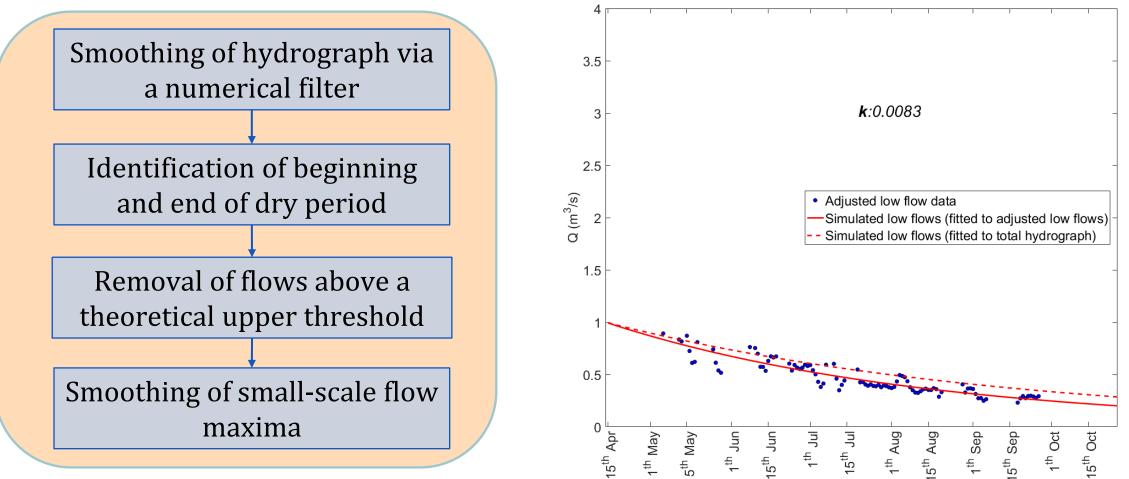
- **Reference time horizon**: April 15th to October 15th
- Adjusted low flows: Estimated on the basis of dry-period hydrograph, after filtering and removal of flow maxima.
- **Initial discharge**, q_{0j} : Minimum flow of the first two weeks of April, *a priori* determined according to the observed data.
- Recession parameter, k_j: Inferred through calibration, by fitting eq. (1) to adjusted low flow data of year *j*.

Study areas



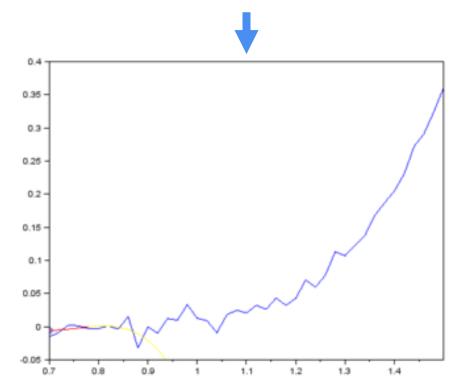
Derivation of adjusted low flow data

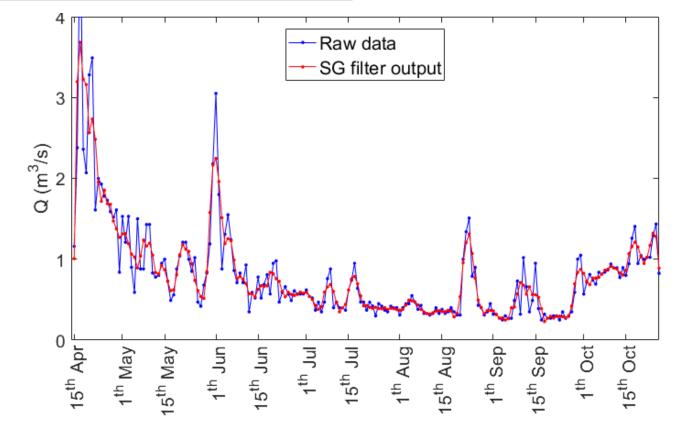
- Real-world dry period hydrograph → rising and recession limbs, individual peaks → underestimation of recession parameter
- Extraction of actual low flows from the total hydrograph \rightarrow adjusted low flows



Step A: Smoothing of total hydrograph

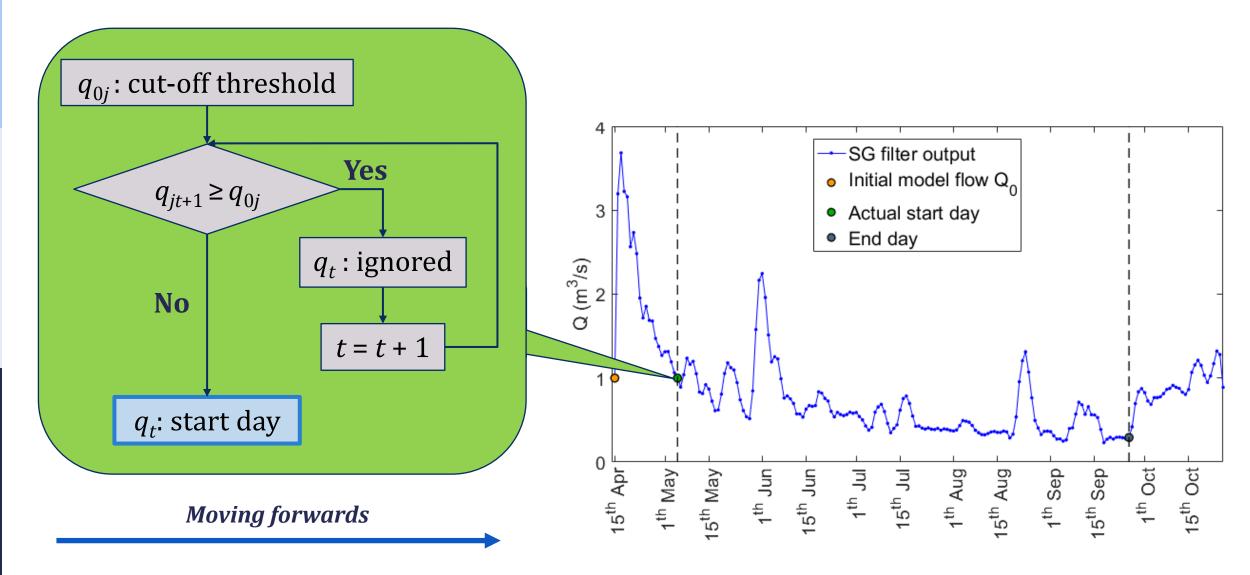
- Savitzky & Golay (1964) numerical filter
- □ 3rd order polynomial
- Fitting period *n* = 59 days
- Moving polynomial fit to 2*n*+1 neighboring points



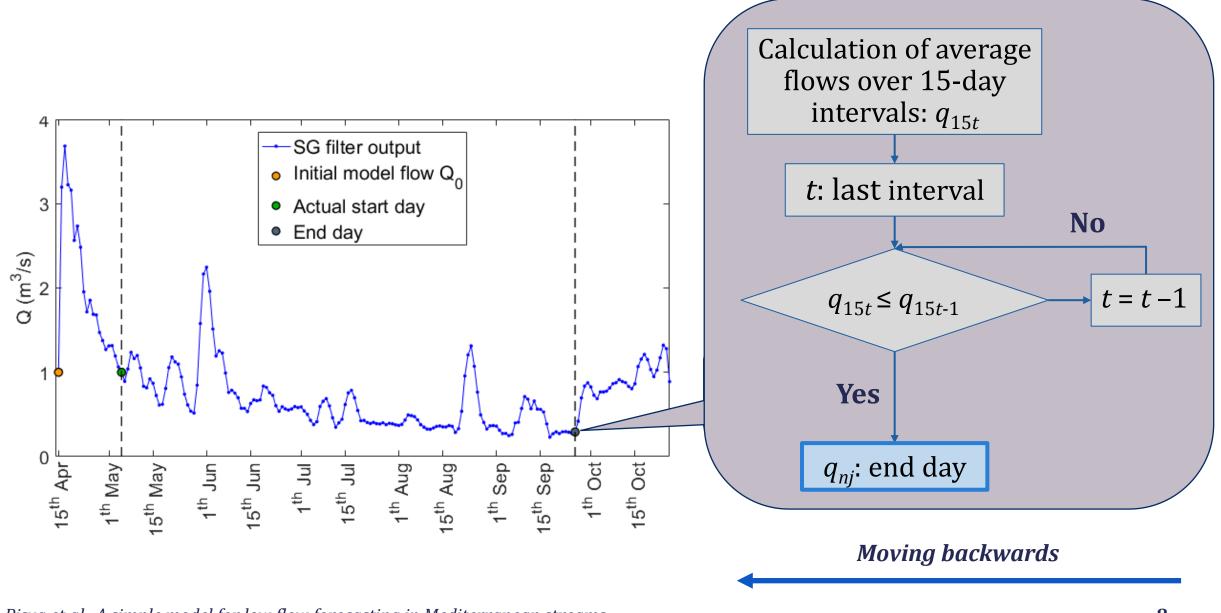


- Increase of the signal-to-noise ratio without significant signal distortion.
- □ Filters both large and small-scale fluctuations.
- Resemblance to weighted MA schemes

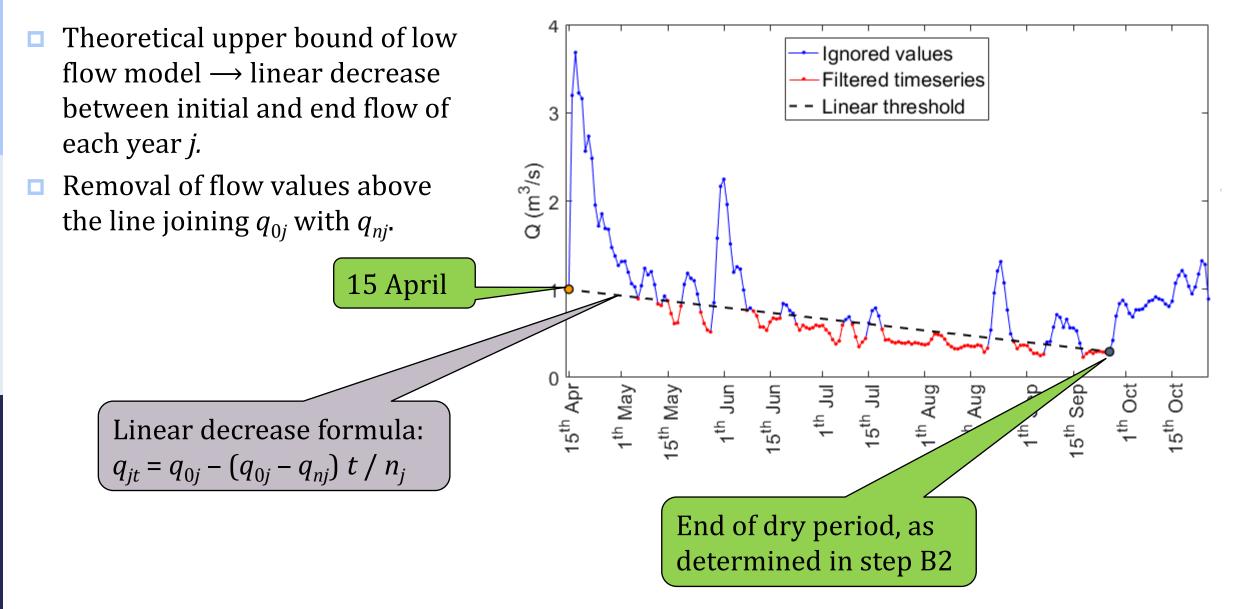
Step B1: Identification of start day



Step B2: Identification of end day

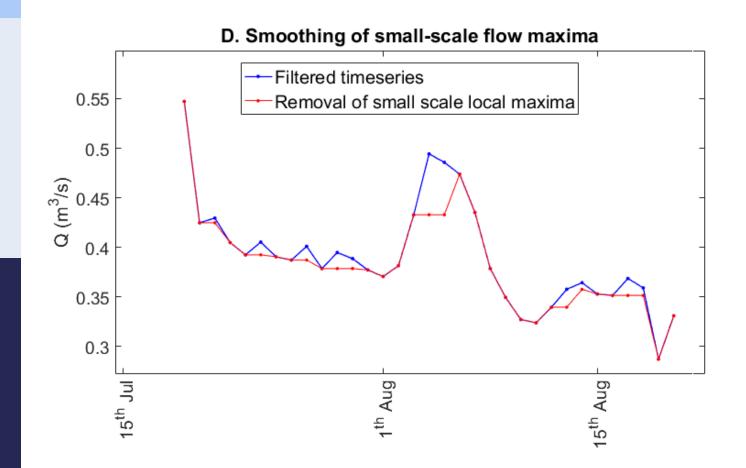


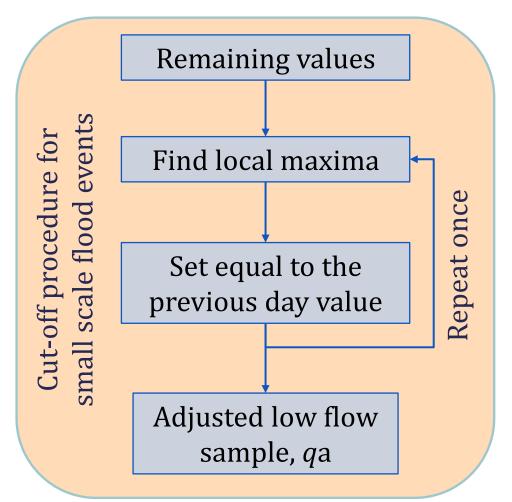
Step C: Removal of flows above theoretical upper threshold



Step D: Smoothing of small-scale flow maxima

- Remaining local flow maxima are reduced
- Adjusted sample *q*a: non-continuous, much less values than the full dry-period sample of length *n_i*





Model calibration

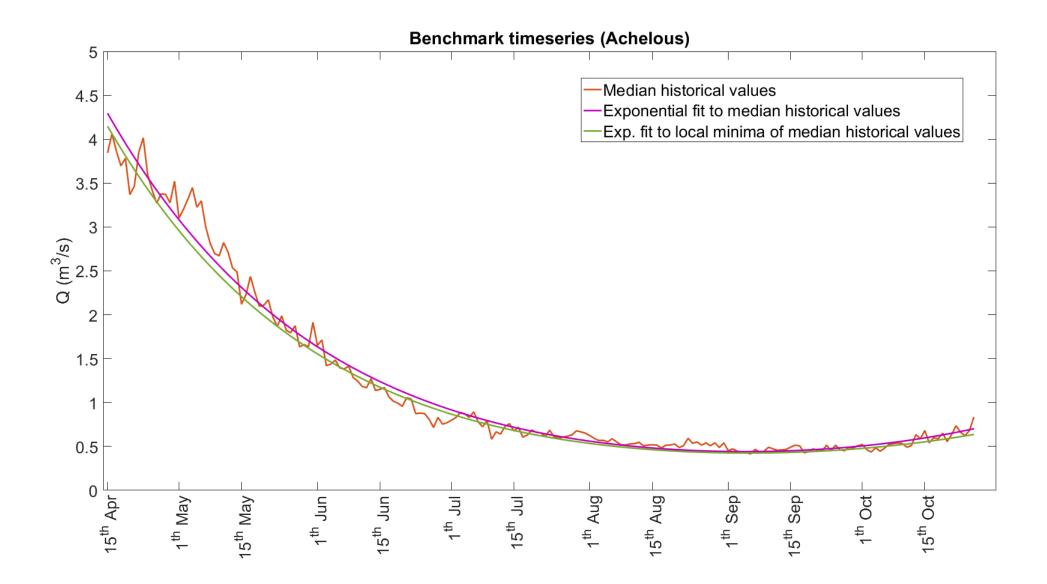
- Shortcomings of traditional NSE metric:
 - Comparison of simulated flows against the average flow provides unrealistically high model performance.
 - The systematic flow decrease is non stationary.

Objective function: modified expression of efficiency (MEF)

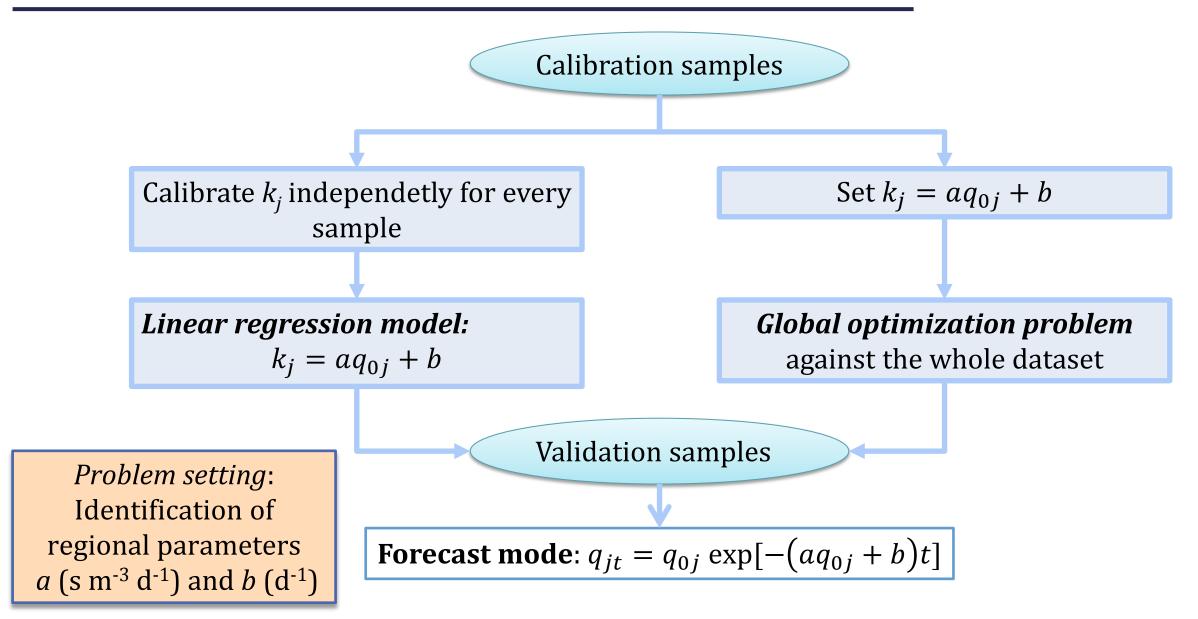
$$MEF = 1 - \frac{\sum \{qa_{jt} - q_{0j} exp(-k_jt)\}^2}{\sum \{qa_{jt} - Benchmarck_t\}^2}$$

- **Benchmark**: mean flow value or median of each individual day
- Stepwise exponential functions fitted to:
 - daily means
 - daily medians
 - **lower envelope of medians** → most representative of the river regime

Benchmark timeseries example

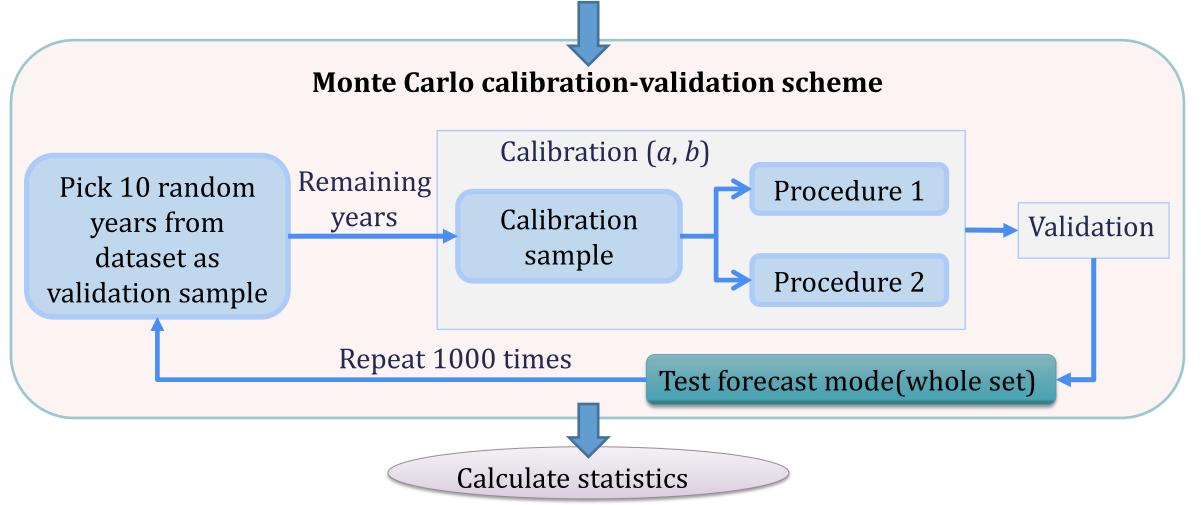


Model formulation in forecast mode

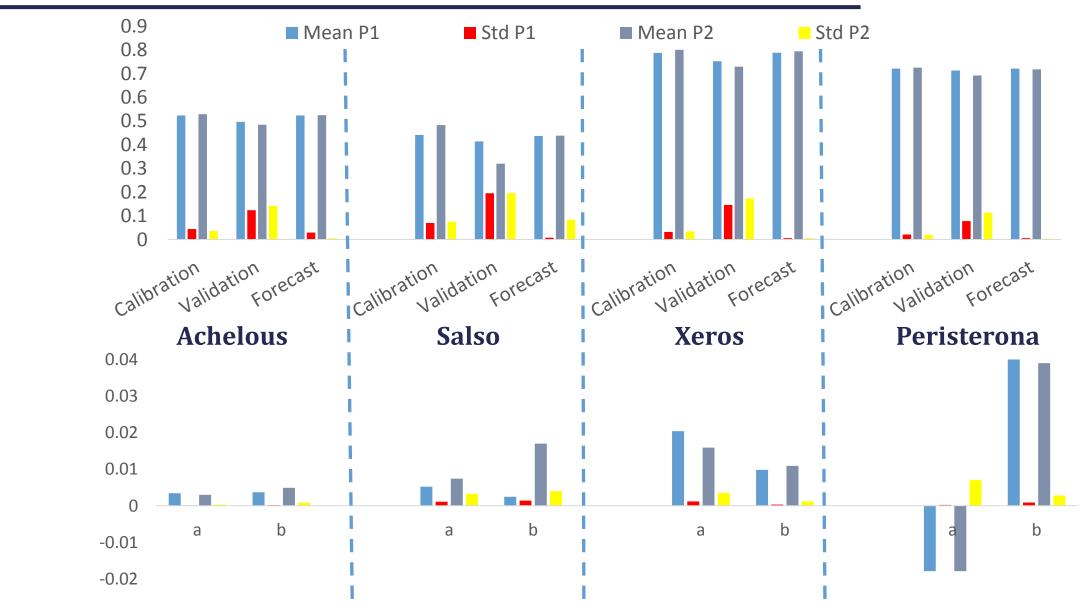


Uncertainty analysis

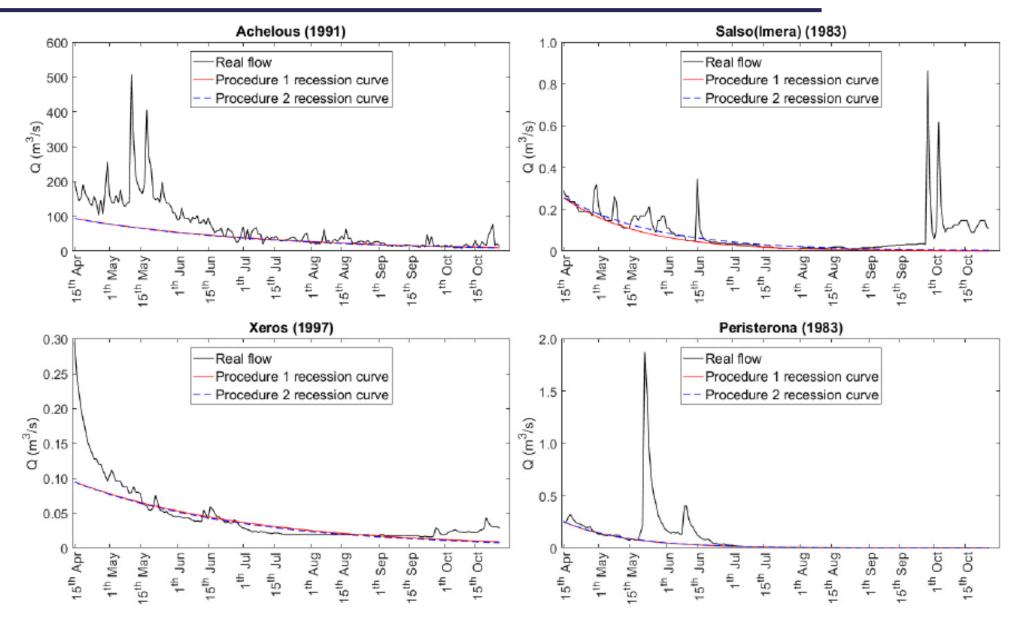
Shortcoming of the classical **calibration-validation paradigm**: Dependency of the model performance on the length and time window of the data sample.



Results procedure 1 (P1) and 2 (P2)



Forecast examples



Conclusions

- Low flow dynamics in Mediterranean rivers can be well approximated using the linear reservoir concept.
- The proposed methodology is suitable for river basins of a wide range of spatial extent, producing both permanent and intermittent runoff during the dry period.
- The strong advantage of the model is its parsimony, in terms of
 - data requirements
 - parameters
- The model is easy to apply in an operational context, since after calibration the sole input is the starting flow q₀, which is easily observable.



Low flows on the Lower Darling River